

- DISEÑE LA BARRA MÁS TRACCIONADA EN PERFIL CIRCULAR
- TODAS LAS BARRAS ESTÁN EN EL MISMO PLANO
- DISEÑE LA ROTULA EN "E", UTILIZANDO UN PERNO AISL N. CIZALLE DOBLE
- DIBUJE SU SOLUCIÓN CONSTRUCTIVA EN ESE APOYO
- DETERMINE ESPESOR MÍNIMO DE LAS PLANCHAS DE UNIÓN
- DISEÑE LA SOLDADURA CON LA QUE EL PERFIL BC SE UNE A LA PLANCHA EN "C"

$$\alpha = \arctg\left(\frac{3,6}{1,8}\right) = 60,642^\circ$$

$$\beta = \arctg\left(\frac{1,8}{1,2}\right) = 56,31^\circ$$

$$\gamma = \arctg\left(\frac{1,8}{3,0}\right) = 30,964^\circ$$

1 - Reacciones



$$\sum F_x = 0 \quad - A_x + E_x = 0 \Rightarrow A_x = E_x$$

$$\sum F_y = 0 \quad E_y - 10 = 0 \Rightarrow E_y = 10 \text{ t}$$

$$\sum M_A = 0 \quad + E_x \cdot 3,2 + E_y \cdot 1,8 - 10 \cdot 6 = 0$$

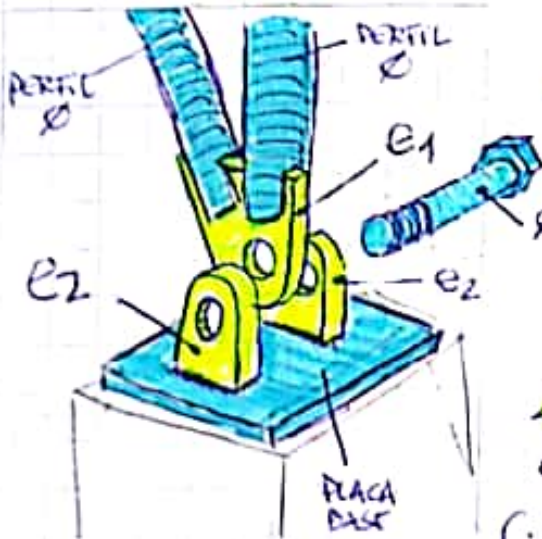
$$E_x = \frac{60 - 10 \cdot 1,8}{3,2} = 13,125 \text{ t}$$

$$\therefore R_E = \sqrt{13,125^2 + 10^2} = 16,5 \text{ t}$$

2 - Diseño rótula "E"

Según tablas perno $\phi 33$ $A = 8,55 \text{ cm}^2$ (AISI N en C.O)

$$\therefore f_u = \frac{16.500}{8,55} = 964,9 \frac{\text{Kg}}{\text{cm}^2} < 1.050 \frac{\text{Kg}}{\text{cm}^2} \text{ OK}$$



3 - DISEÑO DE PLANCHAS DE UNIÓN

Y SOLUCIÓN CONSTRUCTIVA
Acero planchas A270ES

TODOS LOS PERFILES SERÁN SUDADOS A LAS PLANCHAS CON SOLDADURA DE FILETE

4 - Espesor de planchas e1 y e2

carga de aplastamiento = 16.500 Kg

$$\therefore \frac{16.500}{2 \cdot 700} = 6,11 \Rightarrow e_1 = 14 \text{ mm}$$

$$f_{cp} = \frac{16.500}{3,3 \times 1,4} = 3571,4 \frac{\text{Kg}}{\text{cm}^2} < 3.645 \frac{\text{Kg}}{\text{cm}^2} \text{ OK}$$

para e2

$$\frac{16.500}{2 \cdot 1700} = 3,06$$

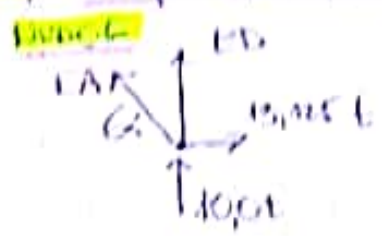
$$\Rightarrow e_2 = 8 \text{ mm}$$

para e1

$$f_{cp} = \frac{16.500}{2 \cdot 33 \cdot 0,8} = 3125 \frac{\text{Kg}}{\text{cm}^2} < 3.645 \frac{\text{Kg}}{\text{cm}^2} \text{ OK}$$

5. - Fuerzas en las barras

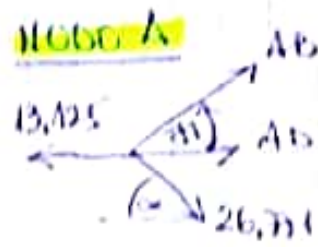
$\alpha = 60,147^\circ$



$\sum F_x = 0$
 $-EA \cdot \cos(60,147) + 13,125 = 0$
 $EA = +26,771 \text{ Kg}$ (Traction)
 $\sum F_y = 0$
 $EB + 10,0 + 26,771 \cdot \sin(60,147) = 0$

$EB = -10,0 - 23,333 = -33,333 \text{ Kg}$
 (Compression)

NOVO A

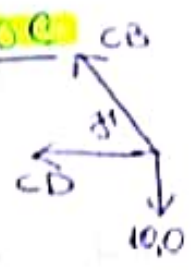


$\beta = 30,964^\circ$
 $\sum F_y = 0$
 $AB \cdot \sin(30,964) - 26,771 \cdot \sin(60,147) = 0$

$AB = 45,350,7 \text{ Kg}$ (Traction)

$\sum F_x = 0$
 $-13,125 + 45,350,7 \cdot \cos(30,964) + 26,771 \cdot \cos(60,147) + AD = 0$
 $AD = -38,887,8 \text{ Kg}$ (Compression)

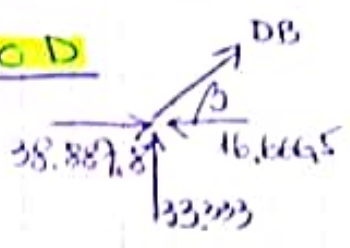
NOVO C



$\sum F_y = 0$
 $CB \cdot \sin(30,964) - 10,0 = 0 \Rightarrow CB = +19,436,4 \text{ Kg}$
 (Traction)

$\sum F_x = 0$
 $-19,436,4 \cdot \cos(30,964) - CD = 0$
 $CD = -16,666,5 \text{ Kg}$ (Compression)

NOVO D



$\beta = 56,31^\circ$
 $\sum F_y = 0$
 $DB \cdot \sin(56,31) + 33,333 = 0$
 $DB = -40,064,2 \text{ Kg}$
 (Compression)

$\sum F_x = 0$
 $-9,62 \approx 0$

\therefore Barra más traccionada es $AB = 45,350,7 \text{ Kg}$

6. - Diseño Barra + traccionada

$\beta = 30,964 < 45^\circ$

$L_{AB} = \sqrt{1,8^2 + 3,0^2} = 3,5 \text{ m}$

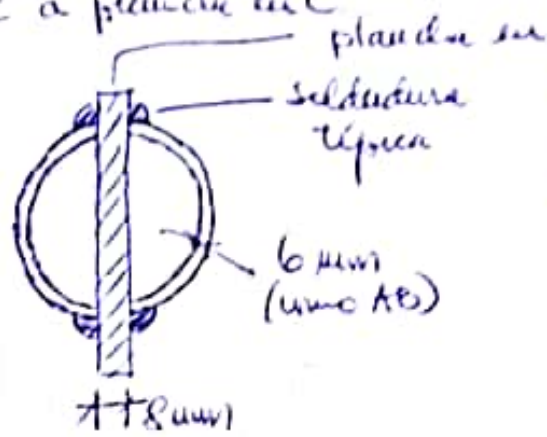
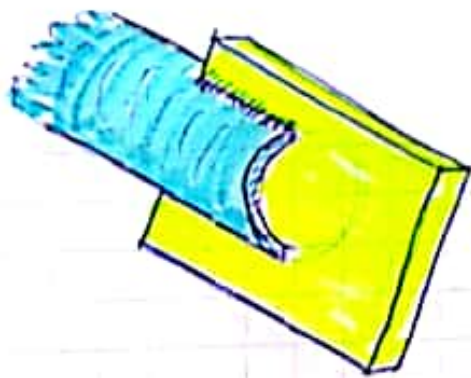
requiere de esbeltez $\frac{L}{i} \leq 240 \Rightarrow \frac{350}{240} = 1,46 \text{ cm}$

$\frac{F}{A} \leq F_e \leq 0,6 \cdot 2700 \Rightarrow A \geq \frac{45350,7}{1620} = 28,0 \text{ cm}^2$

selecciono $\odot 162 \times 6 \text{ mm}$ pero = 23,08 Kg/m
 $A = 29,41 \text{ cm}^2$ $r = i = 5,5 \text{ cm}$

$\frac{L}{i} = \frac{350}{5,5} = 63,6 < 240 \text{ Kg}$ $f_e = \frac{45,350,7}{29,41} = 1542,0 \frac{\text{Kg}}{\text{cm}^2} < 1620 \frac{\text{Kg}}{\text{cm}^2}$

7.- Diseña la soldadura de a planche en C



$$e = 8 \text{ mm} \\ \frac{1}{\cos \alpha} \\ 5$$

Electrodo E6011

$$\Delta_{\text{min}} = 5 \text{ mm} \\ \Delta_{\text{max}} = 8 - 2 = 6 \text{ mm}$$

$$f_u \leq F_u \begin{cases} 1270 \text{ Kg/cm}^2 \text{ soldadura} \\ 10000 \text{ Kg/cm}^2 \text{ metal base} \end{cases}$$

$$\therefore \text{elijo } a = 6 \text{ mm} \Rightarrow a_{ef} = 0,6 \times 0,707 \\ = 0,4242 \text{ cm}$$

$$N^{\circ} \text{ ardores} = 4 \quad F = 19.436,4 \text{ Kg}$$

$$\frac{F}{a_{ef}} \leq 1.080$$

$$\frac{19.436,4}{4 \cdot 0,4242 \cdot L_{ef}} \leq 1.080 \Rightarrow L_{ef} = 10,61 \text{ cm}$$

$$L_{\text{real}} = 10,61 + 2 \cdot 0,6 = 11,81 \text{ cm} \Rightarrow 12 \text{ cm}$$

$$\therefore L_{ef} = 12 - 1,2 = 10,8 \text{ cm}$$

$$f_u = \frac{19.436,4}{4 \cdot 0,4242 \cdot 10,8} = 1000,6 \frac{\text{Kg}}{\text{cm}^2} < 10800 \frac{\text{Kg}}{\text{cm}^2}$$

TABLA EVALUACION

1	10	DTOS	7,0
2	15	✓	10,5
3	15	✓	10,5
4	15	✓	10,5
5	20	✓	14,0
6	10	✓	7,0
7	15	✓	10,5
TOTAL	100	PTOS	70 PTD